AEE10/2018

National Council of Science Museums 33, Block-GN, Sector-V, Bidhannagar, Kolkata-700091

Question paper for recruitment of Assistant Executive Engineer 'CIVIL'

Time: 3 Hours			Total: 100 Marks
I) Tick (/) the correct	answer:		(30x1 = 30 marks)
1. Introducing a man my grandfather." How	to her husband, a wo	oman said, "His brother I to this man?	r's father is the only son of
a) Mother	b)Aunt	c)Sister	d) Daughter
2. 16 (81) 25	36 (168) 49	64 (?)	18 (
a)121	b)184	c)250	d)287
3. 10. If 5@6=61 and	8@10=164,then 7@9)=?	
a)125	b)63	c)130	d)32
	language if the word be coded in that lang		s 'LSPAPG', then how will
a) OYIWXY	b) OYIXYW	c)IYXYWO	d.)YXWYOI
	climbing up a tree 2		hops 3ft. and slips back 2ft.
a)21 hours	b)12 hours	c) 18 hours	d) 15 hours
6. A watch shows 4 hand point?	30 Hrs. If the minute	hand points to east, in	what direction will the hour
A) North-West	b) South-East	c)North-East	d) North
7. In a row of trees, there in the row?	a tree is 34th from le	ft end and 15 th from rig	ght end. How many trees are
a) 47	b) 48	c) 49	d) 50
	ed 'box', 'box' is ca 'bag', what will a chi	그렇게 들어보다 살아보니 경험을 하다 내가 되었다면 하는 경기를 하는 것이 없는 것이 없다.	' is called 'sharpner', and
a) eraser	b)bag	c)pencil	d)sharpener
			8 minutes. At what average me distance in 20 minutes?
a) 68 km/hr	b)44 km/h	c)54 km/hr	d)58km/h

10. Sohan and Moh is in the middle of t students are waiting	an are waiting to get on the line. There are six in the line.	n the school bus. Soh students between Sol	an is sev	renth in line. Mohan Mohan.How many
a)12	b)18	c)25	d)27	
11. The appropriate n	neaning of the proverb	idiom "bolt from the b	olue" is	
a)Thundering	b) A complete surpris	se c) deep in co	lour	d) Very fast.
12. One who is able	to use his right and left	hands equally well:		
a) sinister	b) ambivalent	c.)ambidextrous		d)amateur
13. The boy was cure	edtyphoid.			
a) from	b) for	c) of		d) through
14.Calcutta universit	y is university w	which was established	in the 19	th Century.
a) an	b) the	c) this	7	d) a
15. Which of the spel	lling is correct			
a)Etiquete	b)Ettiquete	c)Etiquette		d)Etiquettee
16. A sentence has be select the one which	een given in Active/ pa best expresses the same	assive voice. Out of the e sentence in Passive /	e four su Active V	aggested alternative, Voice.
"You surprise me"				
a)I am to be surprised	I. b) You are sur	rprised c) I am surpri	sed	d) Me is surprised.
17. Find the correct s	ynonym for the word "	Repercussion"		
a) magnificent	b) reaction	c)acceptance		d) resistance
18. How many pegs of	of wine yest	erday?		
a)you have drank	b) were you drinking	c) did you drink	d) do y	ou drink.
19. A of	camels.			
a) pack	b)caravan	c) swarm		d) pride.
20. Choose the sequer	nce to form the correct	sentence.		
It has been established	d that			
A: Einstein was				
B: although a great sc	ientist			

C: weak in arithmeti	c		
D: right from his sch	ool days		
a)DCAB	b)BACD	c)BADC	d)CBAD
21. Which among t devastating blaze?	he following iconic	structures almost bu	rnt down recently after
a)The Colosseum b)	Wembley stadium c)	Westminister Abbey	d)Notre Dame Cathedral
22. Which of the follo	owing country has host	ed the 2018 Winter Ol	ympic?
a)North Korea	b)Japan	c)USA	d)South Korea
23. When is the Inter	rnational Museum Day	observed every year?	
a)18 th May	b) 10 th May	c)24 th May	d) none of the options.
24.At present, how n	nany States and Union	Territories does India	comprise of?
a) 27 & 6	b)28 & 5	c)29 & 7	d)30 & 8
25.The maximum str	ength of the House of t	he People as per the C	onstitution of India is
a)475	b)543	c)550	d)552
26. Which of the following	owing country is not a	member of BRICS gro	oup of nations.
a)Brazil	b)South Africa	c)Saudi Arabia	d)India
27.In which year was	Mother Teresa awarde	ed the Nobel Peace Pr	ize?
a)1975	b)1979	c)1985	d)1990
28. Who is the curren	t Secretary-General of	the United Nations?	
a)Antonio Guteress	b) Kofî A. Annan	c)Ban Ki-Moon	d)none of the options.
29.Which is the higher of India.	est Wartime gallantry a	award given to individ	uals for service to Republic
a)Ashoka Chakra	b)Param Vir Chakra	c)Maha Vir Chakra	d)Kirti Chakra
30.Which country wil	ll host the FIFA World	Cup in the year 2022.	
n)Mexico	b) Germany	c) Japan	d) Qatar

II) Tick (\checkmark) the co	rrect answer:		(30x1=30 marks)
1) Minimum period	d before striking of fo	rmwork for R.C.C slab	spanning upto 4.5 meter is
a) 3 days	b) 7days	c)14 days	d)21 days
2) The recommend	ed range of slump for	Tremie concrete is bet	ween
a) 75-100 mm	b)100-125mm	c)125-175mm	d) 200-250mm
effect of tempera	ture ,creep & shrinka	erm) in RCC beams/sl ge measured from the nbers should not norma	abs due to all loads including as cast level of the supports of ally exceed
a)span/125	b)span/200	c)span /250	d)span /375
4)Minimum reinfornot be less than	rcement in RCC slab	when high strength d	eformed bars are used should
a)0.08%	b)0.10%	c)0.15%	d) 0.12%
5) The maximum segregation.	permissible free fal	I of concrete may be	taken as to avoid
a) 1.5 meter	b)1 meter	c)2 meter	d)none of the above
6) A column is cal dimension is	lled a long column if	the ratio of the effect	ive length to the least lateral
a)≥3	b) ≥6	c)≥12	d) ≥10
7) The maximum sp as per IRC specifica	pacing of contraction ation is	joint in unreinforced ce	ement concrete rigid pavement
1)2.5 mtr	b)4.5 mtr	c)3.5 mtr	d)6 mtr
3)Which of the pave	ement has high flexur	al strength	
)Rigid pavement	b)Flexible paveme	nt c)WBM road	d)all the above
)A sand having atisfactory concrete	fineness modulus m e.	ore than wil	I be unsuitable for making
)2.6	b)2.9	c)3.0	d)3.2
0) When I cm on praction of the scale	olan drawing represen	ts 10 meter on the grou	and, the representative
)1/10	b)1/100	c)1/1000	d)1/10000

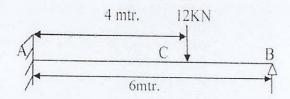
11)The vertical dis	tance between any two	consecutive contour li	nes is called
	nt b)Horizontal equiva		
12) When several o	contours coincide, it in	dicates a	
a)Vertical cliff	b) Valley	c) Ridge	d)Overhanging cliff
13) A first class bri hours immersion in	ck should not absorb w	vater more than of	its dry weight after 24
a)5%	b)15%	c)20%	d)30%
14)The construction	of temporary structure	e required to support ar	unsafe structure is called
a)Underpinning	b)Scaffolding	c)Jacking	d)Shoring
15)The type of bond is called	d in a brick masonry co	onsisting of alternate co	urse of header and stretcher
a)Flemish bond	b)English bond	c)Herringbone bond	d)None of above
16)The unit weight	of brickwork (Brick ma		
a) 1500 kg/m ³	b) 1850 kg/m ³		d) 2050 kg/m ³
17)The range for perwith population above	r capita minimum dem ve 100000 together wit	and of water for domes h full flushing system i	tic use for communities
a)150 -200 lphd	b)250 -300lphd	c)300 - 400 lphd	d)100 -150 lphd
18) In routine vertical working load.	al load test of RCC pile	e the maximum test load	d applied is times the
a)1.2 times	b)1.5 times	c)2.5 times	d) 3 times
19)'N' value of SPT	test in the range of 30-	50 indicates	
a) Very loose soil	b) Loose soil	c) Dense soil	d)None of the above
20)Depth (D) to Wic	dth (B) ratio for shallov	v foundation is general	
a) ≤l	b) ≤2	c) ≤3	d) ≤5
21)As per IS: 875 (Panalysis is given by	art 3) ,Design wind sp	eed (Vz) at any height	'z' in m/sec for wind load
a)0.5V _b k ₁ k ₂ k ₃	b)V _b k ₁ k ₂ c) 3.5	V _b k ₁ k ₂ k ₃	d) V _b k ₁ k ₂ k ₃

22)As per IS code ,t principal direction is		force or Design seismi	e base shear (V _b) along any
a) A _h W	b) A _h W/3	c) A _h W/2	d) none of the above
where W=Seismic w	eight of the building &	A _h = Design horizont	al acceleration spectrum.
23)For determining to per latest IS 1893 co		e country is classified i	nto seismic zones as
a) 5	b) 4	c) 3	d)6
24) The relationship rigidity (EI) is given		rvature (R),Bending	Moment (M) and flexural
a) R=M/EI	b)EI=R/M	c)M=EI/R	d)none of the above
25)Diameter of a bol	t hole is usually taken a	as	
a) nominal diameter d)none of the above	of bolt b) nominal di	ameter +1.5mm c)no	ominal diameter +3 mm
26) The rise and fall	method of leveling pro	vides complete check	on
a) Backsight	b)Intermediate sight	c)Foresight	d)all the above
27) In direct tension	lap length including an	chorage value in reinf	orcement shall be
a) 2L _d or 30 Ø which	never is greater b) leas	st of 3L _d or 24 Ø	c) $2L_d$ d) L_d
where L _d is the devel	opment length & Ø is	the dia of bar.	
28) The short term st	atic modulus of elastic	ity of concrete (E c) ca	n be assumed as
a) 4000 √f ek	b) 5000 √f ck	c)5000 f _{ck}	d) 6000 √f _{ck}
where f ck is the chara	acteristic strength of co	ncrete.	
29) The approximate	weight of each 600mi	m x 600 mm x 10 mm	thick vitrified tiles is
a)7 .5 kg	b)5 kg	c)10 kg	d)2.5kg
			grade reinforced concrete nominal maximum size is
a)250kg/m ³	b) 275kg/m ³	c) 300kg/m ³	d)350kg/m ³

III) Answer any ONE (a) or (b):

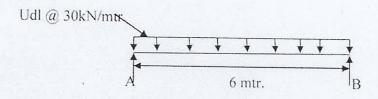
(15 Marks)

(a) Find the support reactions at A & B and also draw the Bending Moment & Shear Force diagram for the propped cantilever beam shown below.



- (b) Write short notes on:
- 1) Initial load test on RCC pile.
- 2) Bearing capacity of soil.
- 3) Non-Destructive Tests (NDT) for Concrete.
- IV) Design the simply supported RCC beam given below as singly reinforced using Limit State Method of design as per IS 456(2000). Consider M20 grade concrete & Fe415 grade steel.

(25 Marks)



Given data: - The limiting value of the depth of neutral axis $(x_{u,max/d})$ is 0.48 for Fe 415 grade of steel.

Enclosure --- Page No. 96 & 73 of IS 456: 2000 and page no. 48 & 179 of SP 16 provided for reference.

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ANNEX G (Clause 38.1)

MOMENTS OF RESISTANCE FOR RECTANGULAR AND T-SECTIONS

G-0 The moments of resistance of rectangular and T-sections based on the assumptions of 38.1 are given in this annex.

in this annex. G-1 RECTANGULAR SECTIONS

G-1.1 Sections Without Compression Reinforcement

The moment of resistance of rectangular sections without compression reinforcement should be obtained as follows:

 Determine the depth of netutral axis from the following equation :

$$\frac{x_{\rm u}}{d} = \frac{0.87 \, f_{\rm y} \, A_{\rm st}}{0.36 \, f_{\rm ck} \, b.d}$$

b) If the value of x_a/d is less than the limiting value (see Note below 38.1), calculate the moment of resistance by the following expression:

$$M_{\rm u} = 0.87 \ f_{\rm y} \ A_{\rm st} \ d \left(1 - \frac{A_{\rm st} f_{\rm y}}{b d f_{\rm ck}} \right)$$

c) If the value of x/d is equal to the limiting value, the moment of resistance of the section is given by the following expression:

$$M_{\text{u-lim}} = 0.36 \frac{x_{\text{u, max}}}{d} \left(1 - 0.42 \frac{x_{\text{u, max}}}{d} \right) b d^2 f_{\text{ek}}$$

d) If x_u/d is greater than the limiting value, the section should be redesigned.

In the above equations,

x = depth of neutral axis,

d = effective depth,

f_y = characteristic strength of reinforce-

A = area of tension reinforcement,

 f_{ck} = characteristic compressive strength of concrete,

b = width of the compression face.

M_{u, lim} = limiting moment of resistance of a section without compression reinforcement, and

 $x_{u, \text{max}}$ = limiting value of x_u from 39.1.

G-1.2 Section with Compression Reinforcement

Where the ultimate moment of resistance of section

exceeds the limiting value, $M_{u, lim}$ compression reinforcement may be obtained from the following equation:

$$M_u - M_{u, lim} = f_{sc} A_{sc} (d - d')$$

where

 M_u , $M_{u, lim}$, d are same as in G-1.1,

 $f_{sc} = \frac{\text{design stress in compression reinforcement corresponding to a strain of}}{\text{ment corresponding to a strain of}}$

$$0.003.5 \frac{\left(x_{u, \max} - d'\right)}{x_{u, \max}}$$

where

 $x_{u, \text{max}}$ = the limiting value of x_u from 38.1,

 A_{ω} = area of compression reinforcement, and

d' = depth of compression reinforcement from compression face.

The total area of tension reinforcement shall be obtained from the following equation:

$$A_{\mathfrak{s}\mathfrak{l}} = A_{\mathfrak{s}\mathfrak{l}\,1} + A_{\mathfrak{s}\mathfrak{l}\,2}$$

where

 A_{u} = area of the total tensile reinforcement,

 A_{vil} = area of the tensile reinforcement for a singly reinforced section for $M_{v, lim}$, and

$$A_{sc2} = A_{sc} f_{sc} / 0.87 f_{y}.$$

G-2 FLANGED SECTION

G-2.1 For $x_e < D_r$ the moment of resistance may be calculated from the equation given in G-1.1:

G-2.2 The limiting value of the moment of resistance of the section may be obtained by the following equation when the ratio $D_f I d$ does not exceed 0.2:

$$M_{u} = 0.36 \frac{x_{u,max}}{d} \left(1 - 0.42 \frac{x_{u,max}}{d} \right) f_{ck} b_{w} d^{2}$$
$$+ 0.45 f_{ck} \left(b_{f} - b_{w} \right) D_{f} \left(d - \frac{D_{f}}{2} \right)$$

mhara

 $M_{u} x_{u,max}$, d and f_{ck} are same as in G-1.1,

 b_f = breadth of the compression face/flange,

 b_{w} = breadth of the web, and

 $D_{\rm f}$ = thickness of the flange.

Table 19 Design Shear Strength of Concrete, τ_c , N/mm² (Clauses 40.2.1, 40.2.2, 40.3, 40.4, 40.5.3, 41.3.2, 41.3.3 and 41.4.3)

100 A,		Concrete Grade							
	M 15	M 20	M 25	M 30	M 35	M 40 and above			
(1)	(2)	(3)	(4)	(5)	(6)	(7			
≤ 0.15	0.28	0.28	0.29	0.29	0.29	0.30			
0.25	0,35	0.36	0.36	0.37	0.37	0.3			
0.50	0.46	0.48	0.49	0.50	0.50	0.5			
0.75	0.54	0.56	0.57	0.59	0.59	0.64			
1.00	0.60	0.62	0.64	0.66	0.67	0.6			
1.25	0.64	0.67	0.70	0.71	0.73	0.7			
1.50	0.68	0.72	0.74	0.76	0.78	0.7			
1.75	0.71	0.75	0.78	0.80	0.82	0.8			
2.00	0.71	0.79	0.82	0.84	0.86	0.8			
2.25	0.71	0.81	0.85	0.88	0.90	0.93			
2.50	0.71	0.82	0.88	0.91	0.93	0.9:			
2.75	0.71	0.82	0.90	0.94	0.96	0.98			
3.00 and above	0.71	0.82	0.92	0.96	0.99	1.0			

NOTE — The term A_i is the area of longitudinal tension reinforcement which continues at least one effective depth beyond the section being considered except at support where the full area of tension reinforcement may be used provided the detailing conforms to 26.2.2 and 26.2.3

Table 20 Maximum Shear Stress, τ_{c max}, N/mm² (Clauses 40.2.3, 40.2.3.1, 40.5.1 and 41.3.1)

Concrete Grade	M 15	M 20	M 25	M 30	M 35	M 40 and above
t _{c max} , N/mm ²	2.5	2.8	3.1	3.5	3.7	4.0

c) Inclined stirrups.

Where bent-up bars are provided, their contribution towards shear resistance shall not be more than half that of the total shear reinforcement.

Shear reinforcement shall be provided to carry a shear equal to $V_u - \tau_e$ bd The strength of shear reinforcement V_{us} shall be calculated as below:

a) For vertical stirrups:

$$V_{\rm us} = \frac{0.87 \, f_{\rm y} \, A_{\rm sv} d}{s_{\rm v}}$$

 For inclined stirrups or a series of bars bent-up at different cross-sections:

$$V_{\rm us} = \frac{0.8.7 \, f_{\rm y} \, A_{\rm sv} d}{s_{\rm us}} \, \left(\sin \alpha + \cos \alpha \right)$$

 For single bar or single group of parallel bars, all bent-up at the same cross-section:

$V_{us} = 0.87 f_{v} A_{sv} \sin \alpha$

where

 A_{rv} = total cross-sectional area of stirrup legs or bent-up bars within a distance s_v .

s_v = spacing of the stirrups or bent-up bars along the length of the member,

τ, = nominal shear stress,

 τ_c = design shear strength of the concrete,

b = breadth of the member which for flanged beams, shall be taken as the breadth of the web b_w.

f_y = characteristic strength of the stirrup or bent-up reinforcement which shall not be taken greater than 415 N/mm²,

 α = angle between the inclined stirrup or bent- up bar and the axis of the member, not less than 45°, and

d = effective depth.

TABLE 2 FLEXURE — REINFORCEMENT PERCENTAGE, p. FOR SINGLY REINFORCED SECTIONS

fck = 20 N/mm1

			fy, N/mn	1		11111		Sy, N/	mm³		and the special division.
M _w /M	1	260			500	Mulbd ³ , N/mm ²	240	250	415	480	500
0-30 0-35 0-40 0-45 0-50	0.146 0.171 0.196 0.222 0.247	0.140 0.164 0.188 0.213 0.237	0.085 0.099 0.114 0.128 0.143	0-073 0-086 0-098 0-111 0-123	0-070 0-082 0-094 0-106 0-119	2·22 2·24 2·26 2·28 2·30	1·253 1·267 1·281 1·295 1·309	1·203 1·216 1·230 1·243 1·256	0.725 0.733 0.741 0.749 0.757	0.627 0.633 0.640 0.647 0.654	0.602 0.608 0.615 0.621 0.623
0·55 0·60 0·65 0·70 0·75	0·272 0·298 0·324 0·350 0·376	0.262 0.286 0.311 0.336 0.361	0·158 0·172 0·187 0·203 0·218	0·136 0·149 0·162 0·175 0·188	0·131 0·143 0·156 0·168 0·181	2·32 2·34 2·36 2·38 2·40	1·323 1·337 1·351 1·365 1·380	1·270 1·283 1·297 1·311 1·324	0·765 0·773 0·781 0·790 0·798	0.661 0.668 0.675 0.683 0.690	0.635 0.642 0.648 0.655 0.662
0.80 0.85 0.90 0.95 1.00	0·403 0·430 0·456 0·483 0·511	0·387 0·412 0·438 0·464 0·490	0·233 0·248 0·264 0·280 0·295	0·201 0·215 0·228 0·242 0·255	0·193 0·206 0·219 0·232 0·245	2·42 2·44 2·46 2·48 2·50	1·394 1·408 1·423 1·438 1·452	1·338 1·352 1·366 1·380 1·394	0.806 0.814 0.823 0.831 0.840	0.697 0.704 0.711 0.719 0.726	0.669 0.676 0.683 0.690 0.697
1.05 1.10 1.15 1.20 1.25	0·538 0·566 0·594 0·622 0·650	0·517 0·543 0·570 0·597 0·624	0·311 0·327 0·343 0·359 0·376	0.269 0.283 0.297 0.311 0.325	0·258 0·272 0·285 0·298 0·312	2·52 2·54 2·56 2·58 2·60	1:467 1:482 1:497 1:512 1:527	1:408 1:423 1:437 1:451 1:466	0·848 0·857 0·866 0·874 0·883	0·734 0·741 0·748 0·756 0·764	0·704 0·711 0·719 0·726 0·733
1:30 1:35 1:40 1:45 1:50	0.678 0.707 0.736 0.765 0.795	0.651 0.679 0.707 0.735 0.763	0·392 0·409 0·426 0·443 0·460	0·339 0·354 0·368 0:383 0·397	0·326 0·339 0·353 0·367 0·382	2·62 2·64 2·66 2·68 2·70	1·542 1·558 1·573 1·588 1·604	1·481 1·495 1·510 1·525 1·540	0·892 0·901 0·910 0·919 0·928	0·771 0·779 0·786 0·794	0·740 0·748 0·755
1.55 1.60 1.65 1.70 1.75	0.825 0.855 0.885 0.916 0.947	0.792 0.821 0.850 0.879 0.909	0.477 0.494 0.512 0.530 0.547	0°412 0°427 0°443 0°458 0°473	0°396 0°410 0°425 0°440 0°454	2·72 2·74 2·76 2·78 2·80	1.620 1.636 1.651 1.667 1.683	1.555 1.570 1.585 1.601 1.616	0·937 0·946 0·955		
1·80 1·85 1·90 1·95 2·00	0.978 1.009 1.041 1.073 1.106	0.939 0.969 1.000 1.030 1.062	0.565 0.584 0.602 0.621 0.640	0·489 0·505 0·521 0·537 0·553	0·469 0·484 0·500 0·515 0·531	2·82 2·84 2·86 2·88 2·90	1·700 1·716 1·732 1·749 1·766	1.632 1.647 1.663 1.679 1.695			
2·02 2·04 2·06 2·08 2·10	1·119 1·132 1·145 1·159 1·172	1·074 1·087 1·099 1·112 1·125	0·647 0·655 0·662 0·670 0·678	0·559 0·566 0·573 0·579 0·586	0·537 0·543 0·550 0·556 0·562	2·92 2·94 2·96 2·98	1·782 1·799 1·816 1·833	1·711 1·727 1·743 1·760			
2·12 2·14 2·16 2·18	1·185 1·199 1·212 1·226 1·239	1·138 1·151 1·164 1·177 1·190	0.685 0.693 0.701 0.709 0.717	0·593 0·599 0·606 0·613 0·620	0·569 0·575 0·582 0·588 0·595						

Norz — Blanks indicate inadmissible reinforcement percentage (see Table E).

TABLE 62 SHEAR - VERTICAL STIRRUPS

Values of Vuld for two legged stirrups, kN/cm.

STIKKUP SPACINO,		DIAME	0 N/mm² Ter, mm			$f_{y} = 4$	15 N/mm ²	
cm	6	8				Diam	TER, MM	
5 6 7	2·460 2·050	4.373	6.833	12	6	8	10	12
7 8 9	1·757 1·537	3.644 3.124 2.733	5·694 4·881	9-839 8-200 7-028	4·083 3·403 2·917	7·259 6·049 5·185	9·452 8·102	16·334 13·611 11·667
10	1.367	2·429 2·186	4·271 3·796 3·416	6·150 5·466 4·920	2·552 2·269 2·042	4·537 4·033	7·089 6·302	10-208 9-074
11 12 13	1·118 1·025	1-988 1-822	3·106 2·847	4.472	1.856	3.630	5·671 5·156	8·167 7·424
14 15	0-946 0-879 0-820	1.682 1.562 1.458	2·628 2·440 2·278	4·100 3·784 3·514 3·280	1·701 1·571 1·458 1·361	3·025 2·792 2·593	4·726 4·363 4·051	6·806 6·286 5·833
16 17 18 19	0-769 0-723 0-683 0-647	1·366 1·286 1·215 1·151	2·135 2·010 1·898 1·798	3-075 2-894 2-733	1·276 1·201 1·134	2·420 2·269 2·135 2·016	3·781 3·545 3·336 3·151	5·445 5·104 4·804 4·537
20	0-615	1-093	1-708	2·589 2·460	1·075 1·020	1.910 1.815	2-985 2-836	4·298 4·083
2.5 30 35	0·492 0·410- 0·351	0·875 0·729 0·625	1·367 1·139 0·976	1·968 1·640	0·817 0·681	1-452 1-210	2·269 1·890	3·267 2·722
40 45	0-307 0-273	0-547 0-486	0-854 0-759	1406 1-230 1-093	0-583 0-510 0-454	1-037 0-907 0-807	1.620 1.418 1.260	2:333 2:042 1:813

TABLE 63 SHEAR - BENT-UP BARS

Values of Vus for singal bar, kN

BAK	fy = 250	N/mm ^s	$f_y = 415 \text{ N/mm}^2$		
DIMETER, mm	α == 45°	α = 60°	a == 45°	a = 60°	
10	12·08	14·79	20·05.	24·56	
12	17·39	21·30	28·87	35·36	
16	30·92	37·87	51·33	62·87	
18	39·14	47·93	64·97	79·57	
20	48·32	59·18	80·21	98·23	
22	58*46	71·60	97·05	118.86	
25	75*49	92·46	125·32	153.48	
28	94*70	115·98	157·20	192:53	
32	123*69	151·49	205·32	251:47	
36	156*54	191·73	259·86	318:27	

Note - a is the angle between the bent-up bar and the axis of the member,